NOTES:

- PRECAST SUPPLIER SHALL DESIGN WALL
 PANEL CONNECTIONS FOR THE REACTIONS
 PROVIDED. SEE PRECAST PANEL SCHEDULE
 FOR THE VERTICAL AND HORIZONTAL PANEL.
 REINFORCEMENT TO BE USED. PRECAST SUPPLIER
 SHALL VERIFY THAT THE REINFORCEMENT SPECIFIED
 IN THE SCHEDULE IS ADEQUATE FOR PANEL
 INSTALLATION AND FOR RESISTING CONNECTION
 FORCES.
- 2. SEE SHEET S2-522 FOR PRECAST PANEL TYPES.
- 3. SEE ARCHITECTURAL WALL ELEVATIONS FOR PANEL NUMBERS, SIZES, AND OPENING SIZES AND LOCATIONS.

	PRECAST PANEL SCHEDULE						
			PANEL CONNECTIONS				
			R1, R2				
	PANEL	TYPE	(LB/IN)	NOTES			
POWER PLANT	16	Α	98				
	17	Α	94				
	18	Α	212				
	19	Α	174				
	20	Α	193				
	21	С	104	ZONES 2 & 4 COMBINED - V: (8)#4 TOTAL, EQ SPACED, ESW			
	22	С	166	ZONES 2 & 4 COMBINED - V: (6)#4 TOTAL, EQ SPACED, ESW			
	23	С	126	ZONES 2 & 4 COMBINED - V: (7)#4 TOTAL, EQ SPACED, ESW. GRATING BACK-UP NOT REQ'D			
	24	В	146	ZONES 2 & 4 COMBINED- V: (8)-#4 TOTAL, EQ SPACED. ESW			
	25	В	181	ZONES 2 & 4 COMBINED - V: (10)#4 TOTAL, EQ SPACED, ESW			
	26	В	160	ZONES 2 & 4 COMBINED - V: (12)#4 TOTAL, EQ SPACED, ESW			
	27	В	195	ZONES 2 & 4 COMBINED - V: (6)-#4 TOTAL, EQ SPACED, ESW			
	28	D	158	ZONES 2 & 4 COMBINED - V: (6)-#4 TOTAL, EQ SPACED, ESW. GRATING BACK-UP NOT REQ'D			
	29	D	181	ZONES 2 & 4 COMBINED - V: (10)#4 TOTAL, EQ SPACED, ESW			
	30	D	282	ZONES 2 & 4 COMBINED - V: (16)#4 TOTAL, EQ SPACED, ESW. SOLID ZONES REQ'D			
	31	С	178	ZONE 2 - V: (2) #4 ESW			
	32	С	155	ZONE 2 - V: (2):#4 ESW			
	33	Α	98				
	34	Α	146	V: (3)#4			

NOTES

- 1. THE PANEL REINFORCEMENT WAS DESIGNED FOR THE SITE PERFORMANCE REQUIREMENTS. CHANGES BY THE CONTRACTOR IN PANEL DIMENSIONS, CONNECTION LOCATIONS, VERTICAL REINFORCEMENT, HEADER AND SILL REINFORCEMENT, OR MATERIALS MAY REQUIRE THE CONTRACTOR TO RETAIN THE SERVICES OF A QUALIFIED BLAST ENGINEER AND SUBSEQUENT REVIEW BY THE GOVERNMENT. THIS MAY RESULT IN CHANGES TO THE DESIGN, INCLUDING CONNECTION LOAD DEMANDS. ALL SUCH CHANGES SHALL BE IN ACCORDANCE WITH ACI 318 EXCEPT AS NOTED OR APPROVED, AND SHALL RESULT IN NO ADDITIONAL COST TO THE GOVERNMENT.
- 2. PANEL SIZING IS BASED ON A CONCRETE 28 DAY COMPRESSION STRENGTH (fc) OF 5000 PSI, ASTM A615 GRADE 60 REINFORCEMENT, AND FULLY LOADED OPENINGS.
- 3. PANEL REQUIRED REINFORCEMENT AND PANEL CONNECTION LOADS ARE BASED ON A 3-2-3 (CONCRETE-INSULATION-CONCRETE) COMPOSITE SANDWICH PANEL. LIFTING HARDWARE SHALL BE SIZED AND REINFORCEMENT SHALL BE VERIFIED OR INCREASED BY THE MANUFACTURER AS REQURED FOR ERECTION LOADS (SEE NOTE 1). CHANGES IN HORIZONTAL REINFORCEMENT (EXCLUDING HEADERS AND SILLS) AND THE ADDITION OF SOLID ZONES WILL NOT ADVERSELY AFFECT EXISTING DESIGN.
- 4. PRECAST DESIGN ASSUMES A COMPOSITE SHEAR TIE SYSTEM. THE COMPOSITE SHEAR TIE SYSTEM SHALL BE DESIGNED BY THE MANUFACTURER AND IS BASED ON THE TIE STRENGTH DERIVED FROM ICC AC-320 OR AC-422 AND EITHER THE ULTIMATE FLEXURAL STRENGTH OF THE PANEL OR THE SHEAR FLOW AT THE PANEL'S ULTIMATE FLEXURAL CAPACITY. THE ULTIMATE FLEXURAL STRENGTH OF THE PANEL IS THE LOWER OF EITHER THE CONCRETE CAPACITY OF THE COMPRESSION WYTHE OR STEEL CAPACITY OF THE TENSION WYTHE. THE SHEAR FLOW OR INTERFACE SHEAR DEMAND IS DERIVED USING ELASTIC CONCEPTS AND A UNIFORM LOAD THAT WOULD EQUATE TO THE ULTIMATE FLEXURAL CAPACITY OF THE PANEL.
- 5. PANEL CONNECTION DESIGN (PANEL TO STRUCTURE) IS THE RESPONSIBILITY OF THE MANUFACTURER AND SHALL BE DESIGNED USING
- 5. INCIDENTAL OPENINGS NOT CAPTURED IN PANEL DESIGNS SHALL FOLLOW ACI STANDARD OF PRACTICE.
- 7. PROVIDE MINIMUM REINFORCEMENT AS FOLLOWS UNO:
 - V: #4@12" O.C., ESW (EACH STRUCTURAL WYTHE)
 - H: #3@18" O.C., ESW

NOTES:

- PRECAST SUPPLIER SHALL DESIGN WALL
 PANEL CONNECTIONS FOR THE REACTIONS
 PROVIDED. SEE PRECAST PANEL SCHEDULE
 FOR THE VERTICAL AND HORIZONTAL PANEL
 REINFORCEMENT TO BE USED. PRECAST SUPPLIER
 SHALL VERIFY THAT THE REINFORCEMENT SPECIFIED
 IN THE SCHEDULE IS ADEQUATE FOR PANEL
 INSTALLATION AND FOR RESISTING CONNECTION
 FORCES FORCES. 53-522
- 2. SEE SHEET \$2-522 FOR PRECAST PANEL TYPES.
- 3. SEE ARCHITECTURAL WALL ELEVATIONS FOR PANEL NUMBERS, SIZES, AND OPENING SIZES AND LOCATIONS.

	DDFCACT DANEL COLUMN					
	i T		PANEL CONNECTIONS	PRECAST PANEL SCHEDULE		
			MIN R1, R2			
	PANEL	TYPE	(LB/IN)	NOTES		
	. 1	Α	126.3 4.7			
	2	Α	147.5 [37			
	3	Α	199-2 373	, 10TAL		
	4	Α	126:3 (47	1011		
E	5	С	126.3 176	ZONES 2 & 4 COMBINED - V: (6) - #4, ER SPACED, ESW		
₹	6	С	1 17. 5 [67			
一面	7	С	147-5 (37	70:		
띪	- 8	С	147:5 156	ZONES 284 COMBINED - V. (B)-#4 TOTAL BY SPACED, FSW		
1821	9	С	1 23. 3 168	7 7 62 3/// 62/		
HE	10	В	126.3 /53	ZONES 224 COMBINED - V: (4) - H4 TOTAL BY SPECED ESW		
၂ပ	11	В	117.5 42			
	12	С	126.3 202	ZONE 4 -V: (3) - #4 FSIM		
	13	С	1 26.3 343			
	14	С	147:5 163	ZONE 2-V! (3)-44 ESW		
	15	С	147.5 203	Zould 4 - 11: (7) - 44 MShA		

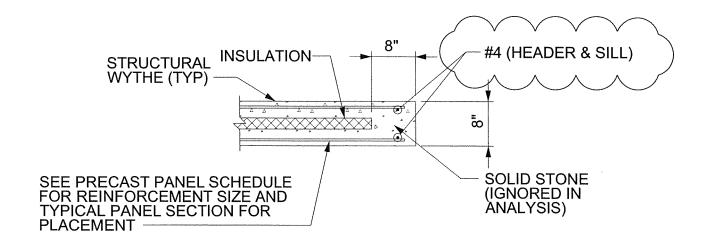
REQ'D SOLID ZONE

- 1. THE PANEL REINFORCEMENT WAS DESIGNED FOR THE SITE PERFORMANCE REQUIREMENTS. CHANGES BY THE CONTRACTOR IN PANEL DIMENSIONS, CONNECTION LOCATIONS, VERTICAL REINFORCEMENT, HEADER AND SILL REINFORCEMENT, OR MATERIALS MAY REQUIRE THE CONTRACTOR TO RETAIN THE SERVICES OF A QUALIFIED BLAST ENGINEER AND SUBSEQUENT REVIEW BY THE GOVERNMENT. THIS MAY RESULT IN CHANGES TO THE DESIGN, INCLUDING CONNECTION LOAD DEMANDS. ALL SUCH CHANGES SHALL BE IN ACCORDANCE WITH ACI 318 EXCEPT AS NOTED OR APPROVED, AND SHALL RESULT IN NO ADDITIONAL COST TO THE GOVERNMENT.
- 2. PANEL SIZING IS BASED ON A CONCRETE 28 DAY COMPRESSION STRENGTH (fc) OF 5000 PSI, ASTM A615 GRADE 60 REINFORCEMENT, AND FULLY LOADED OPENINGS
- 3. PANEL REQUIRED REINFORCEMENT AND PANEL CONNECTION LOADS ARE BASED ON A 3-2-3 (CONCRETE-INSULATION-CONCRETE) COMPOSITE SANDWICH PANEL. LIFTING HARDWARE SHALL BE SIZED AND REINFORCEMENT SHALL BE VERIFIED OR INCREASED BY THE MANUFACTURER AS REQURED FOR ERECTION LOADS (SEE NOTE 1). CHANGES IN HORIZONTAL REINFORCEMENT (EXCLUDING HEADERS AND SILLS) AND THE ADDITION OF SOLID ZONES WILL NOT ADVERSELY AFFECT EXISTING DESIGN.
- 4. PRECAST DESIGN ASSUMES A COMPOSITE SHEAR TIE SYSTEM. THE COMPOSITE SHEAR TIE SYSTEM SHALL BE DESIGNED BY THE MANUFACTURER AND IS BASED ON THE TIE STRENGTH DERIVED FROM ICC AC-320 OR AC-422 AND EITHER THE ULTIMATE FLEXURAL STRENGTH OF THE PANEL OR THE SHEAR FLOW AT THE PANEL'S ULTIMATE FLEXURAL CAPACITY. THE ULTIMATE FLEXURAL STRENGTH OF THE PANEL IS THE LOWER OF EITHER THE CONCRETE CAPACITY OF THE COMPRESSION WYTHE OR STEEL CAPACITY OF THE TENSION WYTHE. THE SHEAR FLOW OR INTERFACE SHEAR DEMAND IS DERIVED USING ELASTIC CONCEPTS AND A UNIFORM LOAD THAT WOULD EQUATE TO THE ULTIMATE FLEXURAL CAPACITY OF THE PANEL.
- 5. PANEL CONNECTION DESIGN (PANEL TO STRUCTURE) IS THE RESPONSIBILITY OF THE MANUFACTURER AND SHALL BE DESIGNED USING
- 6. INCIDENTAL OPENINGS NOT CAPTURED IN PANEL DESIGNS SHALL FOLLOW ACI STANDARD OF PRACTICE
- 7. PROVIDE MINIMUM REINFORCEMENT AS FOLLOWS UNO:

V: #4@12" O.C., ESW (EACH STRUCTURAL WYTHE)

H: #3@18" O.C., ESW

53-521



TYPICAL HEADER/SILL/JAMB PANEL DETAIL

<u> 52-522</u> <u> 52-522</u>